

**THE EFFECTS OF COARSE AGGREGATES ON  
THE STRENGTH OF CONCRETE USING  
ORDINARY PORTLAND CEMENT.**

**By:**

**OKPANACHI SAMUEL ADAMU  
PGD/AGRIC ENG/2000/2001/152**

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**A THESIS SUBMITTED IN PARTIAL  
FULFILMENT OF THE REQUIREMENTS FOR  
THE AWARD OF POST-GRADUATE DIPLOMA IN  
AGRICULTURAL ENGINEERING (SOIL AND  
WATER ENGINEERING) FEDERAL UNIVERSITY  
OF TECHNOLOGY, MINNA.**

**JUNE 2002**

## CERTIFICATION

This is to certify that, this project was carried out by Okpanachi, Samuel A. in the Department of Agricultural Engineering, Federal University of Technology, Minna.

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**Mr. B.A. Alabadan  
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**Date**

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**Dr. D. Adgidzi  
(Head of Department)**

.....

**Date**

.....

**External Examiner**

.....

**Date**

## DEDICATION

I dedicate this work to God, and my family - Mrs. Ruth Samuel, Ahiaba Samuel, Helen Samuel, Ojonoka P. Samuel, Ebiojo V. Samuel, Eikojonwa J. Samuel and Blessing A. Samuel and my late mother Rebecca Ademu Garbiel Okpanachi.

# CHAPTER ONE

## 1.0 Introduction

Concrete plays a vital role in Agricultural development as are eminent in farm structures like: dams, irrigation drains, wells, roads, culverts etc. It is because of these tremendous importance that concrete plays that has brought our mind to think of the effect of the aggregates on the strength and to also see the step to step manufacturing techniques bearing in mind the standard use of the various elements that compose concrete.

In building farm structures. Concrete is needed in the foundation, the over site (German floor), the lintels, slabs and column. This study looks at the type of aggregates and the concrete mix that could be used for all the various purposes.

In irrigation, which is a very important aspect of Agriculture, it is needed to transport water long distance to improve the soil moisture for the life of plants. These are done through either open or closed channel drain using concrete construction. The concrete must meet all requirements, which include the use of coarse aggregates as an element.

It is therefore important to know the effect these aggregates have on the strength so that we may be able have an effective structure. This is a systematic study of the technology of concrete, its various elements, and the strength so attained after some period of time.

## 1.1 STATEMENT OF PROBLEM

- (i) The study looks at the effect of coarse aggregates on the strength of concrete.
- (ii) This study intends to find out the effect of the duration of curing on the strength of concrete.

## **1.2 AIMS OF THE STUDY**

The major aims of this study is to determine the effect of coarse aggregate on the strength of concrete using Ordinary Portland Cement and specifically aimed at finding out:

- (a) The composition of the various elements and the coarse aggregate to determine their effects on the strength of concrete.
- (b) The effect of duration of curing of the concrete on the strength.

## **1.3 SIGNIFICANCE OF THE STUDY**

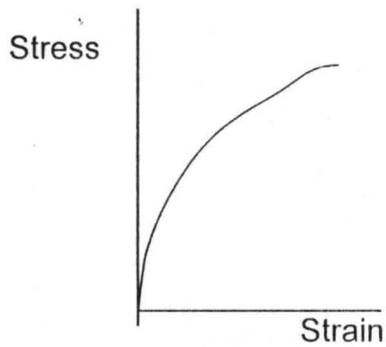
Concrete is a vital element in the strength, durability, deflection, and longevity of a structure, we intend that agriculturists should know the techniques and principles of producing concrete bearing in mind the various elements involved and the batching techniques. That also the appropriate material for concrete production are employed at appropriate time in other not to jeopardize the final strength of the concrete which is at its final at the age of the 28th day

## CHAPTER TWO

### 2.0 Literature Review

#### 2.1 Concrete

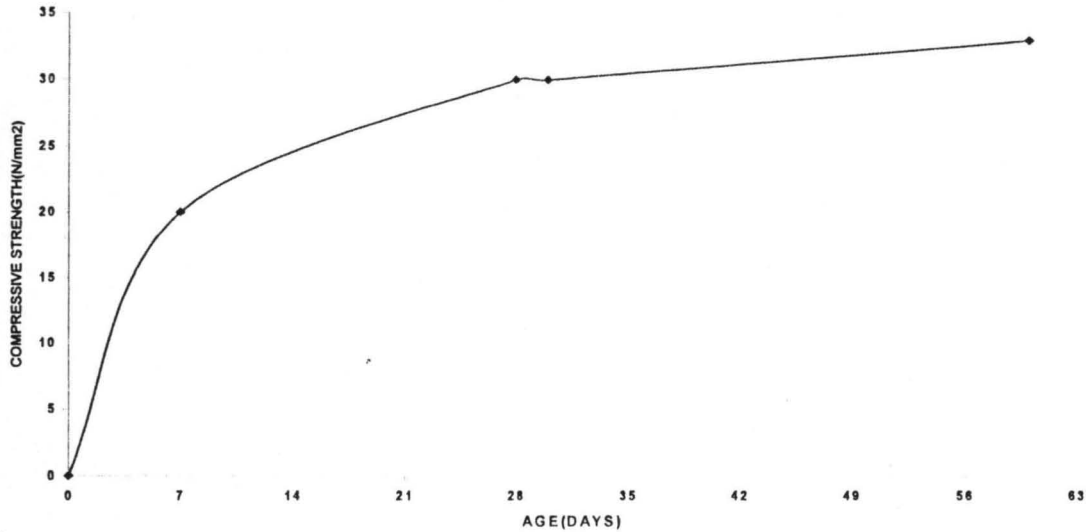
Concrete is a very variable material having a wide range of strength and stress-strain curve. A typical curve for concrete in compression is in the form of the graph.



**Fig. 1. STRESS – STRAIN CURVE FOR CONCRETE IN COMPRESSION**

A load is applied on a cast cured concrete cube, at first the ratio between stress and strain are approximately linear and the concrete behaves almost as an elastic with virtually some expectation of recovery if load is recovered. But as this loading continues beyond this elastic point, there is permanent deformation where recovery is no more possible. Irrespective of the grade of the concrete, it is a recorded fact that concrete generally increases in strength with age during curing to a maximum strength in 28 days as illustrated below.

THE STRENGTH OF CONCRETE WITH AGE



A typical variation in strength of an adequately cured ordinary Portland cement concrete as suggested by Bs 8110 and CP 110, Part 1, 1972 is as shown in tables 1 and 2 below:

**Table 1 Variation in strength of adequately cured concrete**

Duration	Strength N/MM <sup>2</sup>
7 days	20
1 month	30
2 months	33
3 months	35
6 months	36
1 year	37

**Table 2 Grade of concrete**

Grade	Characteristic Strength N/MM <sup>2</sup>	Lowest Grade for Compliance with Appropriate Use
	7.0	Plain concrete
10	10.0	
15	15.0	Reinforced concrete with light weight aggregate
20	20.0	Reinforced concrete with dense aggregate
25	25.0	
30	30.0	Concrete with Post-Technical tendons
40	40.0	Concrete with the tensioned tendons
50	50.0	



**Sources: Table 47 CP 110 Part 1 1972**

Bs 8110 condemns the use of concrete beyond the curing strength of that of 28 days.

The selection of a type of concrete is mostly governed by the strength required which depend on the intensity of loading and the form and size of structural members. The strength is assessed by crushing, the cubes or a cylinder of concrete made from the designed mix. The type of cement used in concrete production cannot be over-looked. Exposure condition and durability are factors to look out for in the choice of cement. For example, a structure subjected to corrosive action of chemical will trigger the mind to use a denser and higher grade concrete and a blast furnace or sulphate resisting cement being used. So also low-heat cement is used to reduce the heat of hydration.

Rapid hardening is employed when an early strength is required. In this experiment we are concerned with ordinary Portland cement in the production of normal concrete.

Minimum cement content is required in the production of normal concrete to ensure durability under specified condition of exposure and the maximum free water/cement ratio to be used are recorded in the Table 3 below.

**2.3 CP110 Part 1(1972). Minimum Cement Required in Portland Cement Concrete to ensure durability under specified condition or exposure.**

Exposure	Reinforced Concrete				Prestressed Concrete				Plain Concrete			
	Nominal Maximum Size of Aggregate				Nominal Maximum Size of Aggregate (mm)				Nominal Maximum Size of Aggregate (mm)			
	40	20	14	10	40	20	14	10	40	20	14	10
Mild: e.g. completely protected against weather, or aggressive conditions, except for a brief period of exposure to normal weather conditions during construction.	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>
	220	250	270	290	300	300	300	300	200	220	250	270
Moderate: e.g. sheltered from severe rain and against freezing whilst saturated with water. Buried concrete and concrete continuously under water.	260	290	320	340	300	300	320	340	220	250	280	300
Severe: e.g. exposure to sea water, moorland water, living rain, alternate wetting and drying and to freezing whilst wet. Subject to heavy condensation or corrosive times.	320	360	390	410	320	360	390	410	270	310	330	360
Subject to salt used for de-icing (see 6.3.7)	260	290	320	340	300	320	340	240	280	210	210	330
When the maximum free water/cement ratio can be strictly controlled (see 6.3.3) these values may be reduced to:												

## 2.4 REQUIREMENT FOR FRESH CONCRETE

Workability: CP 110 Part 1, 1972 Clause 6:4:2 states that – The workability of the fresh concrete should be such that the concrete is suitable for the condition of handling and placing so that after compaction it surrounds all reinforcements, tendons and ducts and completely fills the formwork.

Workability should be assessed by means of slump test, compaction factor test or VB consisto-meter test as appropriate. The above code was strictly adhered to as slump test was conducted for each experience.

## 2.5 PRODUCTION OF CONCRETE

Provision should be made to protect cement before use to prevent accidental mixing of different types – (CP 110, 1972 – 6.7.2).

2.5.1 **Aggregates:** separate fine and coarse aggregates should be used except for grades 7, 10 and 15 where all in aggregate may be used. Separate storage facilities with adequate provision of drainage should be provided for each different size of aggregate used.

For grades of concrete other than 7, 10 and 15, the grading of each size of aggregate from each pit, quarry or other source of supply should be determined (sieve analysis). - (CP 110, Part 1, 1972, 6:7:3).

## 2.6 Batching And Mixing

This recommend that; the quantity of cement, the quantity of fine aggregate and the quantity of the various sizes of coarse aggregate should be measures by weight to be aided by table next page except that aggregate may be measured by volume in the following instances:-

- (1) Concrete of grade 7, 10 and 15 using dense aggregate.
- (2) Concrete of grade 7 and 10 using lightweight aggregate.
- (3) The light - weight coarse aggregate component only in concrete of grade 15 and higher - (CP 110, Part 1, 1972 – 6:7:4).

2.6 CP110 Part 1(1972). Prescribed Mixes for Ordinary Structural Concrete

Concrete Grade	Nominal size of aggregate (mm)	40		20		14		10	
	Workability	Medium	High	Medium	High	Medium	High	Medium	High
	Limits to slump that May be expected (mm)	50 – 100	100 – 150	25 – 75	75 – 125	10 – 50	50 – 100	10 – 25	25 – 50
7	Cement (kg)	180	200	210	230	--	--	--	--
	Total aggregate (kg)	1950	1850	1900	1800				
	Fine aggregate (%)	30 – 45	30 – 45	35 – 50	35 – 50				
10	Cement (kg)	210	200	240	260	--	--	--	--
	Total aggregate (kg)	190	1850	1850	1800				
	Fine aggregate (%)	30 – 45	30 – 45	35 – 50	35 – 50				
15	Cement (kg)	250	270	280	310	--	--	--	--
	Total aggregate (kg)	1850	1800	1800	1750				
	Fine aggregate (kg)	30 – 45	35 – 45	35 – 50	35 – 50				
20	Cement (kg)	300	320	320	350	340	380	360	410
	Total aggregate (kg)	1850	1750	1800	1750	1700	1700	1750	1650
	Sand*								
	Zone 1 (%)	35	40	40	45	45	50	50	55
	Zone 2 (%)	30	35	35	40	40	45	45	50
	Zone 3 (%)	30	30	30	35	35	35	40	45
30	Cement (kg)	370	290	400	430	430	470	460	510
	Total aggregate (kg)	1750	1750	1700	1650	1700	1600	1650	1650
	Sand*								
	Zone 1 (%)	35	40	40	45	45	50	50	55
	Zone 2 (%)	30	35	35	40	40	45	45	50
	Zone 3 (%)	30	30	30	35	35	40	40	45

A separate weighing device should be provided for weighing the cement. Alternatively, the cement may be measured by using a whole number of bags in each batch. The amount of water should be measured by volume or by weight.

## ***2.7 Compliance With Specified Requirements***

Clause 6.8.1 of CP 110, Part 1, 1972 specifies that:-

Provided that the Engineer is satisfied that that materials used are in accordance with his specification and with the constituent materials of the concrete and that correct methods of manufacture and practices of handling raw materials and manufactured concrete have been used, the compliance of:-

2.5.4 (1) A designed mix for ordinary structured concrete should be judged by the strength of the compression with the specified characteristic strength, together with the cement content in compression with the specified minimum cement content.

2.5.4 (2) A prescribed mix for ordinary structural concrete should unless otherwise specified, be judged on the basis of the specified mix proportions and required workability. The Engineer must satisfy himself that the concrete is likely to have strength.

A separate weighing device should be provided for weighing the cement. Alternatively, the cement may be measured by using a whole number of bags in each batch. The amount of water should be measured by volume or by weight.

## **2.7 Compliance With Specified Requirements**

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Provided that the Engineer is satisfied that the materials used are in accordance with his specification and with the constituent materials of the concrete and that correct methods of manufacture and practices of handling raw materials and manufactured concrete have been used, the compliance of:-

(1) A designed mix for ordinary structural concrete should be judged by the strength of the compression with the specified characteristic strength, together with the cement content in comparison with the specified minimum cement content.

(2) A prescribed mix for ordinary structural concrete should unless otherwise be specified, be judged on the basis of the specified mix proportions and required workability. The Engineer must satisfy himself that the concrete is likely to have strength.

## **2.8 Strength**

Clause 6:8:2:1 specifies about strength that – the characteristic strength of concrete is that 28 – day – cured strength below which not more than 5% of the test results may be expected to fall.

Compliance with the specified characteristic strength should be judged by tests made on cubes at an age of 28 days unless there is evidence, satisfactory to the Engineer, that a particular regime is capable of predicting the strength at 28 days of concrete test at an earlier age when compliance may be based on the results of such tests alone.

## **2.9**      **Water/Cement Ratio**

The total water/cement ratio of batch of concrete should not exceed the specified maximum value by more than 5% of that value. If maximum water/cement ratio has been specified the ability to comply with that requirement at a suitable level of workability will have been determined by means of trial mixes, provided that the constituent materials and mix proportion are not substantially different from those used in the trial mixes. A maximum water/cement ratio may be judged from workability test as contained on CP 110, Clause 6:8:5.

## CHAPTER THREE

### 3.0 Materials And Methods

#### 3.1. Location:

The experiments were conducted at Abuja at the Dantata and Sawoe Construction Company Abuja. All their available materials from the quarry, the soil and concrete laboratory were used. The company is involved in the construction of "outstanding engineering infrastructure at Gudu District Phase II of Abuja City using grade 25 for all the experiment specifically the samples that were taken were the materials for concreting of bridges.

#### 3.2 . Materials:

The materials that were involved in the experiments were water, ordinary Portland cement (Ashaka cement), washed river sand, crushed stone dust and coarse aggregates from quarry along airport road which complied with BS 1047, BS 882 BS 3797 and 4619.

#### 3.3. Methodology:

The experiments to determine the strength were divided into three basic parts- (a) Sieve analysis (b) Production of concrete (c) Testing for strength.

##### (a) Sieve Analysis:

Aggregates of nominal sizes 0-10mm, 16mm, 19mm, 32mm diameters were employed for the experiments.

The materials so got from the quarry and river for the concrete after using mechanic batch were fetched, hundred grammes of the material passed through a column of BS sieves and the individual pass weight were recorded. The analysis were corresponded with each other to see to their conformity with the road note 4 of "Design of concrete mixes"- (Transport and Road Research Laboratory U.K)



**(b) Production of Concrete**

After the configuration envelope was examined and found to be adequate, the material were batch from the result obtained from the mix design, bearing in mind the minimum cement ratio, to total aggregate weight, minimum cement/water ratio to ascertain the slump level, the relevant aggregate percentages.

Three sets of concrete were cast for each nominal diameter of the coarse aggregates in steel mould of 15cm x 15cm x 15cm. These were left to set after been well vibrated. The steel mould was struck off to reveal the concrete cubes which were submerged into pool of water for curing for seven days.

All the necessary precautions were taken in the production of the concrete, as such we did not allow curing up to the 28<sup>th</sup> day bearing in mind clause 6:8:2 of CP 110.

**(c) Crushing for strength:**

The three cubes for each of the nominal diameters used were crushed in – turn starting from the fine aggregate, to the 16mm, 19mm and the 32mm nominal diameter sizes.

The calculations to determine the cube strength are attached in the appendices.

**Table 6 Sieve Analysis – 16mm**

INDIVIDUAL GRADING (% PASSING)											
S/ NO	SIZE (MM)	TYPE	0.15	0.3	0.60	1.18	2.36	4.76	9.52	12.7	19.00
1	0 – 5	UKES	2.3	8.8	35.5	63.4	83.7	94.8	100	100	100
2	0 – 5	ST. DUST	17.7	44.2	58.8	68.3	79.2	96.3	100	100	100
3	5 – 10	CR. ST	-	-	-	-	0.9	4.6	4.6	100	100
4	10 – 16	CR. ST	-	-	-	-	-	1.3	1.3	37.2	100
5	16 – 22	CR. ST	-	-	-	-	-	-	-	-	

**Table 7 Sieve Analysis for 19mm**

For this experiment just like the former one, a coarse aggregate of 19mm was used.

A fresh sieve analysis is conducted to see grading envelope as recorded below:

**Table 7 Sieve Analysis for 19mm aggregates**

			INDIVIDUAL GRADING (% PASSING)							
S/ No	Size mm	Type	0.15	0.30	0.60	1.18	2.26	4.76	9.52	19.00
1	0 – 5	UKRS	2.9	19.8	36.6	54.0	72.4	90.1	98.0	100
									0	
2	0 – 5	STD	14.1	32.9	51.9	68.6	84.6	99.2	100	100
3	5 – 12	CR. ST	-	-	3.1	3.8	4.9	16.0	77.9	100
4	12 – 19	CR. ST	-	-	-		0.7	1.3	6.1	99.4

**Table 8 Sieve Analysis for 32mm**

This experiment takes the form of the preceding ones only that the largest nominal size of coarse aggregate in use is 32mm diameter.

**Table 8 Sieve Analysis for 32mm aggregates**

			INDIVIDUAL GRADING (% PASSING)								
S/NO	SIZE	TYPE	0.15	0.3	0.6	1.18	2.36	4.75	9.50	19	38.1
1	0-5	UKERS	1.6	11.1	32.0	57.0	80.7	94.0	100	100	100
2	0-5	ST. DUST	14.1	27.7	42.7	59.0	78.1	98.4	100	100	100
3	4-10	CR. ST	-	-	-	10.1	15.7	47.0	99.6	100	100
4	10-16	CR. ST	-	-	-	-	-	6.2	36.7	65.5	100
5	16-32	CR. ST	-	-	-	-	-	0.5	1.00	24.5	100

**COMBINED GRADING**

The Road note 4 "Design of concrete mixes"– (Transport and Road Research laboratory UK) is aimed at corresponding the total percentage design pass to that of established lower and upper limits to determine their suitability for use in concrete work. This influences the tables of the combined grading (Tables 9,10 and 11).

**Table 9 –combined grading envelope for 16mm diameter nominal size**

S/NO	Size	% Allot	0.15	0.30	0.06	1.18	2.36	4.76	9.52	12.70	19.00	SG
1	0-5	25	0.58	2.2	8.9	15.9	19.8	24.0	25.0	25.0	25.0	2.56
2	0-5	12	2.0	5.2	7.1	8.2	9.5	11.6	12.0	12.0	12.0	2.67
3	5-10	31	-	-	-	-	0.3	1.4	19	31.0	31.0	2.67
4	10-16	32						0.4	2.1	12	12	2.7
5	16-22	-										
<b>Total</b>		<b>100</b>	<b>2.58</b>	<b>7.5</b>	<b>16.0</b>	<b>24.1</b>	<b>29.6</b>	<b>37.4</b>	<b>58.1</b>	<b>80</b>	<b>100</b>	

Referring to appendix ii and drawing from table 50, CP110, the percentage allotment for this group must not be exceeded by more than 5%. The allotment is used to multiply out all the percentage passes to obtain the above result.

**Table 10** combined grading envelope for 19mm diameter nominal size.

S/ No	Size mm	% All	0.15	0.30	0.60	1.18	2.36	4.76	9.52	19.00
1	0 – 5	30	0.87	5.94	10.98	16.2	21.72	27.03	29.4	30
2	0 – 5	10	1.4	3.29	51.9	68.6	84.6	9.92	10	10
3	5 – 12	35	-	-	1.00	1.33	1.7	5.6	27.3	35
4	12 – 19	30	-	-	-		0.21	0.39	1.83	29.82
<b>Total</b>			<b>2.27</b>	<b>9.23</b>	<b>17.2</b>	<b>24.4</b>	<b>32.1</b>	<b>43</b>	<b>68.53</b>	<b>105</b>

Appendix iii and table 50 of CP 110 in reference brings about the above grading. The percentage allotment is also multiplied by all the percentage retained.

**Table 11** combined grading envelope for 32mm diameter nominal size.

S/ NO	SIZE	TOTAL% allotment	0.15	0.3	0.6	1.18	2.36	4.75	9.5	19	100
1	0 – 5	18	0.3	2	5.76	10.26	14.53	16.92	18	18	18
2	0 – 5	17	2.4	4.7	7.3	10.0	13.3	16.7	17	17	17
3	4 – 10	15	-	-	-	1.5	2.4	7.1	14.94	15	15
4	10 – 16	25	-	-	-	-	-	1.55	9.2	16.4	25
5	16 – 32	20	-	-	-	-	-	0.1	0.2	4.8	20
<b>TOTAL</b>			<b>2.60</b>	<b>6.7</b>	<b>13.06</b>	<b>30.7</b>	<b>30.23</b>	<b>42.37</b>	<b>59.34</b>	<b>71.2</b>	<b>95</b>

Using appendix iv and table 50 like the other nominal diameters the above table came into been.

**Table 12** Configuration envelope for 16mm nominal diameter.

S/No	0.15	0.30	0.6	1.18	2.36	4.76	9.52	12.70
Max Pass	3	8.1	21	28	35	42	65	100
Total Pass	2.58	7.5	16.0	24.1	29.6	37.4	58.1	80
Max pass	0	3	14	21	28	35	55	100

**Table 13. Configuration envelope for 19mm nominal diameter**

Individual passing	0.15	030	0.60	1.18	2.36	4.76	9.52	19.00
Maximum	1.5	12	27	34	42	48	75	100
Total Passing	1.27	9.23	17.2	24.4	32.1	43	68.53	100
Lowest Passing	0	2	9	16	23	32	45	100

**Table 14 Configuration envelope for 32mm nominal diameter**

S/No	0.15	0.30	0.60	1.18	2.36	4.76	9.50	19	100
Maximum Passing	5	15	22	30	37	47	60	75	100
Total Passing	2.60	6.7	13.06	30	30.23	42.37	59.34	71.2	95
Lowest Passing	2	7	12	18	23	32	44	60	100

## THE CONFIGURATION

After the percentage allotment on the various nominal size of aggregates in table 9,10,6 and 11. The total configurations are found in tables 12, 13, and 14.

Table 12: for 16mm: the combined configuration envelope to road note 4- RTRL, UK was adequate. The total pass for this size fits into the envelope. For example sieve No 0.15 passed 2.58 % of the material, it was needed to be a minimum pass of 0% and a maximum of 3%. Sieve 1.18 passed a total of 24.1% the minimum pass according to configuration should be 21 with a maximum pass of 28% . Sieve 12.70 should pass 100 maximum and within 70% minimum but there was a total pass of 80%.

We say that the 16mm nominal size in this experiment is adequate and conforms to code.

### Table 13.

In this envelope, we observe that sieve 0.15% has maximum allowable pure of 1.5% while the lever limit is 0% but the total pass is 2.27%. This is above the maximum pass for this sieve. But the road rote permit differences in this case up to 5%. Therefore this is ok. Sieve 19.00 passed 100% the lowest and maximum should be 100%. The result for this is equally adequate. The materials is fit for use and has such we proceed to batch and cast the concrete according to design.

### Table 14

This are the results of the percentage allotment for 32mm configuration the total pass for sieve 0.15 was 2.60% and 30% for sieve 1.18 and 71.2% for sieve 19.00. This configuration we say is adequate because it is within the confine of the envelope.

The configuration shows that we could go on and batch the various elements using the mix design.

### Table 15 Crushing strength result at 7 and 28 days

Description	0 – 5mm	16m	19mm	32mm
Age of concrete	7 days	7 days	7 days	7 days
Size of cube	15x15x15	15 x 15 x 15	15 x 15 x 15	15 x 15 x 15
Density	2.19	2.36	2.3	2.13
Weight (kg)	7.38	7.98	7.68	7.18
Crushed fore (N)	288	440	450	478
Strength N/mm <sup>2</sup>	12.8	19.56	19.73	21.26
Crushing Strength for 28 days	19.2	29.34 N/mm <sup>2</sup>	29.54 N/mm <sup>2</sup>	31.89 N/mm <sup>2</sup>
The use of the concrete where applicable	Mortar, blocks, pipes etc.	Lintel, slabs, columns, concrete walls, piers, piles, tunnel etc	Same as 16mm	Foundation, over size concrete, slab (bridges) etc.

## CHAPTER FIVE

### 5.0 CONCLUSION AND RECOMMENDATION

#### 5.1 CONCLUSION

The strengths obtained from the experiment for 7 days were 16mm – 19.56 N/mm<sup>2</sup>, 19mm – 19.73N/mm<sup>2</sup> and 32mm – 21.26N/mm<sup>2</sup>.

Those obtained for 28 days were for 16mm – 29.34N/mm<sup>2</sup>, 19mm – 29.54N/mm<sup>2</sup> and 32mm – 32N/mm<sup>2</sup>.

There exist a relationship between the aggregate and the strength of concrete and also the strength increases with age.

#### 5.2 RECOMMENDATIONS

- a. I recommend the following for use in construction of structures on agricultural and building sites – 0-5mm be used as mortar, blocks, construction of ring pipes, poles etc.
- b. 16mm nominal diameter for lintels, slabs, columns, walls, drains, pile etc.
- c. 19mm nominal diameter – same as 16mm and also for dam-diaphragms, grouting etc.
- d. 32mm could be used for foundation, over site concrete, mass concrete, slabs (bridges) etc.

I would recommend further studies on **THE EFFECT OF CEMENT ON THE STRENGTH OF CONCRETE.**

## APPENDIX I

### **Mixed design for 10mm diameter nominal size**

Referring to tables 48 and 50 for plain concrete of grade 25 for the mix design,

For grade 25 - 10mm aggregate and medium workability =

Cement (Kg) for 10 mm = 360

Total aggregate (Kg) = 1750

Sand:

Zone 1 % = 50% = .5 x 1750 = 875 kg

∴ 360 : 875

1 : 2.4 ≈ 1:2

The step by step crushing are calculate thus:

The size of the cubes - 15cm x 15cm x 15cm

AREA = 15cm x 15cm = 225 cm<sup>2</sup> = 22500 mm<sup>2</sup>

VOLUME = 15cm x 15cm x 15cm = 3375 cm<sup>3</sup> = 3375000 mm<sup>3</sup>

The cubes were weighed and their weights recorded as before

(a) - 7340g

(b) - 7420g

The Density of each of the Cubes are –

(1)  $\frac{\text{Mass}}{\text{Volume}} = \frac{7340}{3375} = 2.17$

(2)  $\frac{7420}{3375} = 2.198 = 2.20$



The Crushed reading

$$(1) \quad 285.75 \text{ KN} = 285750 \text{ N}$$

$$(2) \quad 290.25 \text{ KN} = 290250 \text{ N}$$

$$\therefore \text{The crushed strength (1)} = \frac{285750}{22500} = 12.7 \text{ N/mm}^2$$

$$(2) = \frac{290250}{22500} = 12.9 \text{ N/mm}^2$$

$$\text{The average strength} = \frac{12.7 + 12.9}{2} = \underline{12.8 \text{ N/mm}^2} \text{ for 7 days}$$

## APPENDIX II

### **Mixed design for 16mm and 19mm nominal sizes**

Referring to table 50 for concrete grade 25 and up to 20mm maximum size of aggregate for mild workability the following information will help us to determine the configuration envelope.

Cement (cubic meter) = 320kg.

Total aggt = 1800kg

Sand:-

Zone 1 (%) = 40

2 (%) = 35

3 (%) = 30

Total aggt. For 20mm = 1750kg Density = 2.651 gm/cm<sup>3</sup>

Volume =  $\frac{1750}{2.651} = 660\text{m}^3$

The concrete was cast using the composition of the aggregate so arrived at above. After seven days of curing the cubes so cast for this experiment were crushed and the following readings and calculations obtained:-

Size of cube 15 x 15 x 15 (cm<sup>3</sup>)

Area = 15 x 15cm<sup>2</sup>

Volume = 15 x 15 x 15cm<sup>3</sup>

The cubes were weighed on a balance where the following were revealed

(1) 8.010 Kg

(2) 7.980 Kg

(3) 7.960 Kg

$$\text{Densities of the cubes} = \frac{8010}{3375} = 2.4 \text{ gm/cm}^3$$

$$(2) \quad \frac{7980}{3379} = 2.36 \text{ gm/cm}^3$$

$$(3) \quad \frac{7960}{3375} = 2.36 \text{ gm/cm}^3$$

$$\text{Average} = \frac{8.010 + 7.980 + 7.960}{3} = 7.983 \text{ kg}$$

$$\text{Average Density} = \frac{2.4 + 2.36 + 2.36}{3} = 2.36 \text{ gm/cm}^3$$

Crushing Reading

$$(a) \quad 450 \text{ KN} = 450000 \text{ N}$$

$$(b) \quad 440 \text{ KN} = 440000 \text{ N}$$

$$(c) \quad 430 \text{ KN} = 430000 \text{ N}$$

$$\text{Average} = 440 \text{ KN} = 440000 \text{ N}$$

$$\text{The Characteristic Strength} = \frac{\text{Strength}}{\text{Area}} = \frac{450,000}{22500} = 20 \text{ N/MM}^2$$

$$\frac{440,000}{22500} = 19.56 \text{ N/MM}^2$$

$$\frac{430,000}{22500} = 19.111 \text{ N/MM}^2$$

$$\text{Average Strength} = \frac{20 + 19.56 + 19.111}{3} = 19.56 \text{ N/MM for seven days}$$

3  
24

## APPENDIX III

Bearing in mind these information about the concrete grade 25 with nominal aggregate size 19mm with moderate workability which are:-

Cement: 320 kg/m<sup>2</sup>

Total Aggt 1800

Zone (%) – 40

Zone (%) – 35

Zone (%) – 30

The concrete was cast using the composition of the weights worked out in the design. It was cured for seven days and the crushing test conducted. The following information was handy for use.

Size of cube      15 x 15 x 15 (cm<sup>3</sup>)

Area              15 x 15 (cm<sup>2</sup>)

Dm<sup>3</sup> =      15 x 15 x 15 =      3.375cm<sup>3</sup>

Weights of the cubes

(a) 7585kg

(b) 8001kg

(c) 7450kg

### **Densities**

(a) 7585

3375 =      2.25gm/cm<sup>3</sup>

(b) 8001

3375 =      2.4gm/cm<sup>3</sup>

(c) 7450

3375 =      2.21gm/cm<sup>3</sup>

$$\begin{aligned} \text{Average Wt.} &= \frac{7.585 + 7.450}{3} = 7.678 \\ &= 7.6787\text{kg} \end{aligned}$$

$$\text{Average Density} = \frac{2.25 + 2.4 + 2.21}{3} = 2.3\text{gm/cm}^3$$

Crushing Force

(a) 450KN

(b) 500KN

(c) 400KN

Average Force = 450KN

Crushing Strength  $\text{N/mm}^2$

(a)  $19.74 \text{ N/mm}^2$

(b)  $21.93 \text{ N/mm}^2$

(c)  $17.54 \text{ N/mm}^2$

Average =  $19.73 \text{ N/mm}^2$  for seven days

## APPENDIX IV

For concrete grade 25 of medium workability and limit of slump of 50 – 100

Cement/m<sup>3</sup>            340kg                    340kg

Total Aggregates (kg)    1800

Sand (Aggregates)

Zone 1 (%)            35

Zone 2 (%)            30

Zone 3 (%)            30

$$\text{Aggt Total} = \frac{1750}{2.56} = 660.4$$

Weight of Individual Aggregate

Min cement = 220kg

Max full water cement Ratio = 0.70

Water = 154kg

The concrete was then cast in the cube and cured for seven days after which the following information's covering the crushing was obtained.

Size of cubes            cm<sup>3</sup> = 15 x 15 x 15 = 3375cm<sup>3</sup>

Or 1.5 x 1.5 x 1.5 = 3.375 cm<sup>3</sup>

Weight of cubes        (a) 7500kg

(b) 6950kg

(c) 7100kg

$$\text{Average Wt} \quad \frac{21550}{3} = 7.183\text{kg}$$

$$\text{Density} \quad \begin{array}{l} \text{(a) } \frac{7500}{3375} = 2.2\text{gm/cm}^3 \\ \text{(b) } \frac{6950}{3375} = 2.1\text{gm/cm}^3 \end{array}$$

$$\frac{(C) \ 7100}{3375} = 2.1 \text{ gm/cm}^3$$

Crushing force	(a) 480KN		
	(b) 485KN		
	(c) 470KN	Average =	<u>3478KN</u>

Crushing Strength N/mm<sup>2</sup>

$$(a) \quad \frac{480 \times 1000}{150 \times 150} = 21.33 \text{ N/mm}^2$$

$$(b) \quad \frac{485 \times 1000}{150 \times 150} = 21.56 \text{ N/mm}^2$$

$$(c) \quad \frac{470 \times 1000}{150 \times 150} = 20.89 \text{ N/mm}^2$$

Average 21.26 Nmm<sup>2</sup> for 7 days

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